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# An integrated dynamic model for simulating a full-scale municipal wastewater treatment plant under fluctuating conditions

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#### ABSTRACT

In this study, an integrated dynamic model was developed through combining a mechanistic model, a neural network (NN) model and a genetic algorithm approach, in order to simulate the performance of a full-scale municipal wastewater treatment plant (WWTP) with substantial influent fluctuations. As the base of the integrated model, the mechanistic model was initially established based on the activated sludge model 3 and the EAWAG bio-P module, and was used to generate the residuals for the NN model. The NN model was employed to build the relationship between the input and output variables. The network weights of the NN model were optimized with a genetic algorithm approach. The resulting integrated model was applied to simulate the 5-month performance of a full-scale WWTP with significant influent fluctuations, and the simulation results matched the measured ones of the WWTP well even under influent disturbance conditions. Compared with the individual mechanistic model and NN model, the integrated model was able to capture sufficient residual information to compensate for the inaccuracy of the mechanistic model and improve the extrapolative capability of the NN model. This model established in our work is demonstrated to be an effective and useful tool to simulate the performance of WWTPs.

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#### **1. Introduction**

Activated sludge processes are widely used for the treatment of both municipal and industrial wastewaters. One of main features of wastewater treatment plants (WWTPs) is the fluctuation of influent wastewater quality and quantity, e.g., high temporal variability of inflow and variable concentrations of components [\[1,2\].](#page-6-0) Thus, efficient modeling is essential to improve the monitoring, control and operation of WWTPs [\[3–5\].](#page-7-0)

Activated sludge models (ASM1, 2, 2d, and 3) are able to simulate the complex physical, chemical and biological processes in WWTPs [\[3\]. T](#page-7-0)hey have been proven to be the most effective tools to describe the removal processes of carbonaceous, nitrogenous and phosphate in WWTPs [\[6,7\]. H](#page-7-0)owever, WWTPs are often characterized with a high daily variation in wastewater quality and/or quantity [\[5,8\]. W](#page-7-0)hen a WWTP is subjected to such a variation, the ASM models may exhibit poor predict abilities, and there is a significant mismatch between the model predictions and the measured data [\[9\]. G](#page-7-0)enerally, these models are high-dimensional and contain a large number of kinetic and stoichiometric parameters, which should be determined with plant operation data [\[4,10\]. I](#page-7-0)n addition, the WWTP effluent is not often well predicted by the mechanistic models because of the separation problems of the secondary clarifiers in WWTPs. To sort out this problem, neural network (NN), an appropriate approach, should be introduced into the mechanistic models to improve their simulating capacity.

The NN approach is a powerful and effective tool to deal with problems to extract information out of complex, non-linear data without requiring prior knowledge of the relationships of the process parameters [\[9,11,12\]. B](#page-7-0)ecause of its interpolative capability to capture the effects of some external disturbances, the NN approach has been successfully applied in multivariate non-linear bioprocesses as a useful tool to construct model[\[13–18\]. H](#page-7-0)owever, the NN is typically used as a "black-box" approach, hiding the physics of the model process, and lacks for extrapolative capacity [\[9,19\]. I](#page-7-0)n addition, the gradient algorithm usually used in the back-propagation NN is a local search algorithm and may tend to fall into a local minimum and result in inconsistent and unpredictable performance [\[20\]. G](#page-7-0)enetic algorithm (GA), based on the principles of survival of the fittest strategy, has been proven to be a powerful search and optimization method to solve problems with objective functions that are not continuous or differentiable [\[21,22\]. T](#page-7-0)hus, an intro-

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<span id="page-1-0"></span>

**Fig. 1.** Schematic diagram of the integrated model.

duction of GA might avoid trapping into the local minimum of NN. The NN-GA coupled models have been developed to optimize the various biological processes [\[18,23\].](#page-7-0)

Therefore, in this study an integrated dynamic model was constructed and used to simulate the performance ofWWTPs, especially under influent fluctuating conditions. Our work aimed at: (1) developing a mechanistic model based on ASM3 and EAWAG bio-P module to generate the residuals for the input of the NN; (2) using a back-propagation NN model to establish the relationship between the input and output variables to simulate the dynamic behavior of the WWTP; (3) introducing GA to optimize the NN weight vector; and (4) demonstrating the capacity of the integrated model with a full-scale WWTP as an example.

#### **2. Model development**

To simulate a full-scale WWTP, an integrated model consisted of a mechanistic model, a NN model and a GA approach was established. The mechanistic model was used as the base of the integrated model, and it generated the residuals for the NN model; NN was used to build the relationship between the input and output variables; the network weights of NN were optimized with the GA approach. The schematic representation of this integrated model is shown in Fig. 1.

The mechanistic model was established through a combination of the ASM3 [\[3\]](#page-7-0) and the EAWAG bio-P module [\[24\].](#page-7-0) In the mechanistic model, the active biomass was divided into three categories: heterotrophic organisms  $(X_H)$ , autotrophic organisms  $(X_A)$  and phosphorus accumulating organisms  $(X_{PAO})$ . The intracellular storage product by heterotrophic organisms  $(X_{STO})$ , and the cell-internal storage product  $(X_{\text{PHA}})$  and polyphosphate  $(X_{\text{PP}})$ by the phosphorous accumulating organisms were considered in the model. The inert particular organic material  $(X_I)$  arising from biomass endogenous respiration was also included.

Six dissolved chemical components were considered to be related to the biochemical processes and the reaction stoichiometry, e.g., dissolve oxygen  $(S<sub>0</sub>)$ , readily biodegradable substrate  $(S<sub>S</sub>)$ , ammonium-N ( $S_{NH_4}$ ), nitrite and nitrate-N ( $S_{NO_x}$ ), nitrogen gas ( $S_{N_2}$ ) and inorganic soluble phosphors, primarily orthophosphate  $(S_{PO<sub>A</sub>})$ . For the kinetic processes, hydrolysis process, in which all slowly biodegradable substrate contained in the influent became available to activated sludge, was considered. Eight processes for the heterotrophic organisms  $(X_H)$  and three processes for the autotrophic organisms  $(X_A)$  were included in the mechanistic model [\[3\]. F](#page-7-0)or the phosphorus accumulating organisms  $(X_{PAO})$ , eleven processes were taken into account [\[24\].](#page-7-0) The model was constructed in a matrix format. The kinetic processes of biological conversions adapted from ASM3 [\[3\]](#page-7-0) and EAWAG bio-P [\[24\]](#page-7-0) notation are outlined in [Tables 1 and 2.](#page-2-0) The stoichiometrics of these processes are listed in [Tables S1 and S2. T](#page-6-0)he kinetic coefficients and initial parameters used in the model were adapted from the ASM3 and EAWAG bio-P module [\[3\].](#page-7-0)

Influent flow rate  $Q(t)$ , mixed liquid volatile suspended solid (MLVSS) (t) and the influent concentrations of chemical oxygen demand  $[COD<sub>in</sub> (t)]$ , ammonium  $[NH<sub>4,in</sub><sup>+</sup> (t)]$  and phosphors  $[PO_{4,in}^{3-}(t)]$  as input variables were put into the mechanistic model. As a result, the effluent concentrations of  $\text{COD}_{\text{eff}}(t)$ , NH<sub>4,eff</sub><sup>+</sup> (t),  $\text{PO}_{4,\text{eff}}^{3-}(t)$  were predicted. The residuals,  $e(t)$  of COD, NH<sub>4</sub><sup>+</sup> and  $PO<sub>4</sub><sup>3–</sup>$ , were defined as the differences between the measured data and predicted values by the mechanistic model.

The NN model was used to establish the relationship between the input and output variables. The feed-forward back-propagation NN, consisting of one input layer, one hidden layer and one output layer, was employed. Influent flow rate  $Q(t)$ ; MLVSS(t) in the reactor; the influent concentrations of  $\text{COD}_{\text{in}}(t)$ , NH<sub>4,in</sub><sup>+</sup> (t), PO<sub>4,in</sub><sup>3–</sup> (t), suspended solids  $[SS_{in}(t)]$ , pH<sub>in</sub> (t); the effluent concentrations of COD<sub>eff</sub> (t), NH<sub>4,eff</sub><sup>+</sup> (t), PO<sub>4,eff</sub><sup>3–</sup> (t), SS<sub>eff</sub> (t) were selected as the input variables of the NN model. Since dynamic information could be extracted from the residuals, the residuals  $e(t)$  of COD, NH<sub>4</sub><sup>+</sup> and  $PO<sub>4</sub><sup>3–</sup>$  generated from the mechanistic model were also put into the NN model as the input variables. The network outputs were the residuals  $e(t+1)$  of COD, NH<sub>4</sub><sup>+</sup> and PO<sub>4</sub><sup>3–</sup>. The number of the hidden neurons was determined from the learning curves, which were generated from the recall and generalization process through varying the number of neurons in the hidden layer [\[25\]. T](#page-7-0)he activation function of the neurons of both hidden and output layers were chosen as the sigmoid function,  $f(x) = 1/(1 + e^{-x})$ . Thus, this type of function allowed the network to learn non-linear relationships [\[26\].](#page-7-0)

An "error back-propagation" algorithm was used as the NN learning algorithm, in which the input values of a sample were ini-

### <span id="page-2-0"></span>**Table 1**





tially applied to be learned as the inputs of the NN model. Then, the outputs of the NN model were compared with the given output values of the example and the errors were estimated. The errors were used to analyze which synaptic weight should be modified to reduce the errors for this sample, and the weights and the threshold values of the output and the hidden layers were adjusted using Eqs.  $(1)-(4)$ :

$$
w_{ij}^{t+1} = w_{ij}^t - \eta \frac{\partial E_k}{\partial x_j} y_i + \alpha (w_{ij}^t - w_{ij}^{t-1})
$$
\n(1)

$$
\theta_j^{t+1} = \theta_j^t - \eta \frac{\partial E_k}{\partial x_j} + \alpha (\theta_j^t - \theta_j^{t-1})
$$
\n(2)

$$
w_{hi}^{t+1} = w_{hi}^t - \eta \frac{\partial E_k}{\partial x_i} x_{k,h} + \alpha (w_{hi}^t - w_{hi}^{t-1})
$$
\n(3)

$$
\theta_i^{t+1} = \theta_i^t - \eta \frac{\partial E_k}{\partial x_i} + \alpha (\theta_i^t - \theta_i^{t-1})
$$
\n(4)

where  $x_i$ ,  $x_j$ ,  $y_i$  and  $y_j$  are defined as the input and output values of the hidden layer and the output layer, respectively.  $w_{hi}$  is the connection weight from the hth neuron of the input layer to the ith neuron of the hidden layer, and  $w_{ii}$  is the connection weight from the ith neuron of the hidden layer to the jth neuron of the output layer.  $\theta_i$  and  $\theta_i$  are the threshold values of the hidden layer and output layer, respectively.  $\eta$  is the learning factor, and  $\alpha$  is the momentum factor.

This procedure was repeated for each training example in the training set. The global error was calculated using the equation below:

$$
E = \sum_{k=1}^{k} \sum_{j=1}^{j} \frac{(y_j - d_{k,j})^2}{2}
$$
 (5)

The computation process of the NN model would be terminated by two rules: (1) meeting the setting goal; and (2) reaching the maximum learning time.

The algorithm used in back-propagation is a gradientdescending algorithm. However, the gradient algorithm might tend to fall into a local minimum and result in inconsistent and unpre-

#### **Table 2**

Process kinetic rate equations for the mechanistic model (continuation).



<span id="page-3-0"></span>**Table 3** Measured variables of the full-scale WWTP.

Variable	Symbol	Unit	Mean	$Min-max$
Influent COD	$COD_{in}$	mg/L	198	102-391
Influent $NH_4$ <sup>+</sup>	$NH_{4,in}$ <sup>+</sup>	mg/L	27.3	15.9-40.9
Influent $PO_4^3$ -	$PO_{4,in}^{3-}$	mg/L	1.60	$0.83 - 2.48$
Influent SS	$SS_{in}$	mg/L	97	$44 - 190$
Influent pH	$pH_{in}$		7.41	6.89-7.74
Effluent COD	COD <sub>eff</sub>	mg/L	40.8	$21.6 - 58$
Effluent $NH_4$ <sup>+</sup>	$NH_{4.eff}$ <sup>+</sup>	mg/L	3.0	$0.1 - 10.1$
Effluent $PO_4^3$ -	$PO4.eff$ <sup>3-</sup>	mg/L	1.31	$0.44 - 2.17$
<b>Effluent SS</b>	$SS_{\rm eff}$	mg/L	11	$5 - 19$
<b>MLVSS</b>	<b>MLVSS</b>	mg/L	5774	3909-7983

dictable performance of the NN model [\[23\].](#page-7-0) Thus, to assure the good performance of the integrated model, GA, a global search algorithm, instead of local search algorithm, was introduced into the integrated model to search for the weight vector of the network [\(Fig. 1\).](#page-1-0) The searching mechanisms of the integrated model are briefly described as follows:

- (1) For the integrated model, the initial weights of NN were constructed based on training data sets.
- (2) GA was employed to explore solutions among solution space. Once the GA generated a new solution, the NN model would be used to determine its fitness value for the GA to continue its searching process.
- (3) After the stopping criterion, e.g., generation number, fitness threshold or population convergence, was met, the best solution would be generated by the GA.

After the best weight vectors of the network became available, they were introduced into the integrated model to simulate the WWTP performance.



**Fig. 2.** Simulation results of the effluent COD, NH<sub>4</sub><sup>+</sup> and PO<sub>4</sub><sup>3–</sup> concentrations using the integrated model.



**Fig. 3.** Measured versus predicted values of (A) COD  $(R^2 = 0.850)$ ; (B) NH<sub>4</sub><sup>+</sup>  $(R^2 = 0.930)$ ; and (C) PO<sub>4</sub><sup>3–</sup> ( $R^2 = 0.926$ ) concentrations in the effluent with the integrated model for training data.

#### **3. Materials and methods**

#### 3.1. Description of the Zhuzhuanjing WWTP

The WWTP is located in Hefei City, China, in which wastewater originating from the surrounding city areas is treated. The plant consists of gridirons, primary clarifiers, eight sequencing batch reactors (SBRs). Each SBR has a rectangle configuration and is operated in a fill-and-withdrawal mode. Wastewater is introduced and simultaneously mixed in the fill period; the reactor is aerated in the reaction period; later, the mixed liquor is allowed to settle in the settling period; at the end of each cycle the supernatant is discharged from the reactor. The working volume of each SBR is  $2000 \,\mathrm{m}^3$ . The time allotted for the operating cycle of each SBR is: 30 min for fill, 120 min for reaction, 60 min for settling, and 30 min

<span id="page-4-0"></span>

**Fig. 4.** Measured versus predicted values of: (A) COD  $(R^2 = 0.920)$ ; (B) NH<sub>4</sub><sup>+</sup> ( $R^2$  = 0.792); and (C) PO<sub>4</sub><sup>3–</sup> ( $R^2$  = 0.794) concentrations in the effluent with the integrated model for testing data.

for decanting. The reactor effluent over a 5-month period was collected and measured almost every day [\(Fig. S1\).](#page-6-0) [Table 3](#page-3-0) lists the available calculations from the plant along with their means, maximum and minimum values. An index, daily variation coefficient  $(K_{\text{day}})$ , was introduced to quantitatively evaluate the daily fluctuations of the wastewater quality, which is defined as follows [\[27\]:](#page-7-0)

$$
K_{\text{day}} = \frac{\text{maximum value of the wastewater quality}}{\text{average value of the wastewater quality}} \tag{6}
$$

#### 3.2. Wastewater characterization

Before the simulation with the mechanistic model, the organic matters in the municipal wastewater were fractionized to readily biodegradable organic compounds  $(S<sub>S</sub>)$ , inert soluble organic compounds  $(S_I)$ , slowly biodegradable organic compounds  $(X_S)$  and



**Fig. 5.** Measured versus predicted values of: (A) COD  $(R^2 = 0.128)$ , (B) NH<sub>4</sub><sup>+</sup>  $(R^2 = 0.128)$ , and (C) PO<sub>4</sub><sup>3–</sup> ( $R^2 = 0.001$ ) concentrations in the effluent with the mechanistic model.

inert particular organic compounds  $(X<sub>I</sub>)$ . The biodegradable COD in the influent was the sum of the  $S<sub>S</sub>$  and  $X<sub>S</sub>$  and was calculated from the formula proposed by Grady et al. [\[27\]. T](#page-7-0)he inert fraction  $S_I$  was determined independently and subtracted from the soluble COD to give the fraction  $S_S$ .  $S_I$  was evaluated from the effluent inert COD. The concentrations of  $X_S$  and  $X_I$  were estimated based on the biological oxygen demand (BOD<sub>5</sub>) measurements [\[28\].](#page-7-0)

The experimental data were directly put into the mechanistic model to generate the residuals. However, prior to the NN training, the data sets were normalized to the range of 0–1, in order to have the same order for the variables. After normalization, each variable had a zero of mean and a unitary variance according to its corresponding maximum and minimum values. In this way, the effect of different units was avoided and the variability was reduced. The data preprocessing above was carried out to ensure that the statistical distribution of the values for each net input and output was roughly uniform [\[25\].](#page-7-0)

<span id="page-5-0"></span>

**Fig. 6.** Measured versus predicted values of: (A) COD  $(R^2 = 0.517)$ , (B) NH<sub>4</sub><sup>+</sup>  $(R^2 = 0.703)$ , and (C) PO<sub>4</sub><sup>3–</sup> ( $R^2 = 0.768$ ) concentrations in the effluent with the NN model for training data.

#### 3.3. Analysis and simulation

Measurements of COD,  $NH_4^+$ -N, NO<sub>2</sub><sup>-</sup>-N, NO<sup>3–</sup>-N, PO<sub>4</sub><sup>3–</sup>-P, pH, SS, MLSS and MLVSS followed the Standard Methods [\[29\]. S](#page-7-0)imulation of the mechanistic model was performed with the AQUASIM software package [\[30\].](#page-7-0) For the NN model and integrated model, the computation was performed using software Matlab 7.0 (Mathworks, Natick, USA).

#### **4. Results and discussion**

#### 4.1. Wastewater characterization

As shown in [Fig. S1, t](#page-6-0)he daily variations of influent COD,  $NH_4^+$ and  $PO_4^3$ <sup>-</sup> concentrations are significant. The highest values of



**Fig. 7.** Measured versus predicted values of: (A) COD ( $R^2 = 0.010$ ), (B) NH<sub>4</sub><sup>+</sup>  $(R^2 = 0.039)$ , and (C) PO<sub>4</sub><sup>3–</sup> ( $R^2 = 0.182$ ) concentrations in the effluent with the NN model for testing data.

influent COD,  $NH_4^+$  and  $PO_4^{3-}$  concentrations were detected to be 391, 41 and 2.5 mg/L, respectively, whereas their lowest values were only 102, 16 and 0.8 mg/L, respectively. Thus, the daily variation coefficients of COD,  $NH_4^+$  and  $PO_4^{3-}$  are calculated as 1.97, 1.50 and 1.55, respectively. The high  $K_{\text{dav}}$  values indicated that the fluctuation of wastewater quality of this WWTP was significant.

The fractions of  $S_S$ ,  $S_I$ ,  $X_S$  and  $X_I$  in the total influent COD in this WWTP were calculated. The  $S_S$  fraction in the influent was determined to be 30.1  $\pm$  3.9% of the total COD, and the initial S<sub>I</sub> fraction was estimated to be  $10.3 \pm 2.3\%$  of the total COD. The initial established  $X<sub>I</sub>$  fraction was approximately 11.0% of the total COD and the contribution of  $X_S$  was calculated to be 43.6% of the total COD.

#### 4.2. Simulation results by the integrated model

Before generating the residuals by the mechanistic model, the parameter sensitivity analysis was performed through changing one by one in the simulation. The sensitivity analysis revealed that the maximum specific growth rates of  $X_H$ ,  $X_A$  and  $X_{PAO}$  and the yield coefficients of  $X_H$  and  $X_A$  had great influences on the effluent COD,  $NH_4^+$  and  $PO_4^{3-}$  concentrations. Therefore, the sensitive parameters were calibrated in AQUASIM [\[30\]. U](#page-7-0)sing the calibrated

## <span id="page-6-0"></span>**Table 4**

Root mean square errors for the training and testing data sets of the three models.



model, the residuals between the predicted and measured values were generated.

143 experimental samples were used for the integrated model simulation. The experimental samples were divided into different groups for training and testing sets. 60%, 70%, 80% and 90% of the samples were used for training the NN model, respectively. Correspondingly, 86, 100, 114 and 129 points were used for training, respectively. The remaining samples were used for testing. The root mean square error (RMSE) of the training and the testing sets of the above four groups had little difference (data not shown). Thus, in the subsequent study the former 100 samples were used for training or calibrating the NN model, whereas the remaining 43 samples were employed for model testing or verification.

The number of neurons of the hidden layer was an important parameter for the NN model. A low number of neurons does not provide sufficient parameters to train the neuronal network correctly. On the other hand, an excessive number of neurons leads to overtraining problems and its computational costs become higher [\[25\].](#page-7-0) Thus, the optimal number of the hidden neurons should be carefully selected. In this study, from the absolute errors (scaled) of both recall of the training data sets and the generalization of the testing data sets, the best network structure of 13 neurons in the hidden layer was determined (Fig. S2).

The simulation results with the integrated model illustrated in [Fig. 2](#page-3-0) show that the predicted effluent COD, NH $_4^+$  and PO $_4^{\,3-}$  levels matched the measured ones well. The model was capable of accurately simulating the effects of the variation of influent COD loading. Satisfactory predictions of NH $_4^{\mathrm{+}}$  and PO $_4^{\mathrm{3-}}$  were also observed with respect to the fluctuation of influent concentration. To better evaluate the simulation results, a line of unit slope has been drawn in both [Figs. 3 and 4,](#page-3-0) in which a point situated exactly on the line indicates a perfect fit. [Fig. 3](#page-3-0) shows the comparison between the experimental values for the three output variables and the corresponding values obtained by fitting the integrated model of the training data sets. Comparison between the measured and predicted values for each output variable of the testing data sets is shown in [Fig. 4. T](#page-4-0)his figure shows that the trained integrated model showed no systematic over-prediction or under-prediction with regard to the output variables.

### 4.3. Comparison among the mechanistic model, the NN model and the integrated model

A comparison among the mechanistic model, the NN model and the integrated model was performed. The calibration results of the mechanistic model are illustrated in [Fig. 5.](#page-4-0) Its low coefficient of determination  $(R^2)$  indicates that this model gave the poor prediction results, because some dynamics information was not taken into account in the mechanistic model. For instance, the poor separation in the secondary clarifiers had influence on the effluent quality, but this issue was not addressed in the mechanistic model. The simulation results of the NN model illustrated in [Figs. 6 and 7](#page-5-0) show that although this model could simulate the training data set well, it was over-trained and exhibited the poor extrapolative capacity. By contrast, the integrated model gave the best prediction results and was able to simulate the fluctuation of influent quality and quantity.

The RMSE is another index to evaluate the model performance. Table 4 lists the RMSE values of the training and testing data sets for the three models. The lowest RMSE value of the integrated model suggests that the integrated model gave the best prediction performance again.

The integrated model was capable of accurately simulating the full-scale WWTP performance under the variations of the influent loading and disturbances and describing the influence of separation process in the secondary clarifiers on the effluent quality. This indicates that the residuals contained sufficient relevant information about the dynamic behavior of the system, which could not be taken into account in the mechanistic model itself [\[9\].](#page-7-0) Such a simulation performance can be explained by the fact that the WWTP in this application was an inherently non-linear system with time-varying biological reactions and large influent variations. The integrated model was able to extract the non-linear information from the residuals to compensate for the inaccuracy of the mechanistic model and accordingly could improve the extrapolation ability of the NN model.

#### **5. Conclusions**

- In this work, an integrated dynamic model was established through combining a mechanistic model, an NN model and a GA approach to simulate the performance of a full-scale WWTP with substantial influent fluctuations.
- The mechanistic model was initially established based on the ASM 3 and the EAWAG bio-P module. It was used to generate the residuals for the NN model. The relationship between the input and output variables was built with the NN model, and the network weights of the NN model were optimized with a GA approach.
- The resulting integrated model was applied to simulate the 5 month performance of the WWTP. The simulation results shows that the predicted values by the integrated model matched the measured ones of the plant well under influent disturbance conditions.
- Compared with the mechanistic model and the NN model, the integrated model was able to capture sufficient residual information to compensate for the inaccuracy of the mechanistic model and improve the extrapolative capability of the NN model.
- Considering the great variations in the influent and high disturbances of WWTPs, the integrated model established in our work is an effective and useful tool to simulate such a complex biological system like WWTPs.

#### **Acknowledgements**

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#### **Appendix A. Supplementary data**

Supplementary data associated with this article can be found, in the online version, at [doi:10.1016/j.cej.2010.03.063](http://dx.doi.org/10.1016/j.cej.2010.03.063).

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